

# **The Effect of Water Quality Sources on Concrete Mix Parameters**

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## **1. Introduction**

Fresh or drinkable water is commonly used in the construction process to create concrete components. Recently, various sources of spent water were tested for use in concrete building. Ocean and alkali waters, canal and stream water, textile evaporation, TreatedWastewater, car wash effluent, industrial wastewater, and so on are examples of these. Water from various quality sources was previously used in the creation of building materials. In comparison to potable water, reclaimed wastewater was used in the concrete [1]. Wastewater from power washing machines was utilized in high-strength concrete and its strength was compared to that of freshwater [2]. Concrete strength was also evaluated using textile effluent in compared to conventional water [3]. For concrete development, general and specific treated

wastewater, car wash wastewater, sugary wastewater, saltwater, and treated sewage water were compared to potable and domestic water [4–10]. The impact of water quality on compression strength has been studied [5,6]. As a result, water management, particularly wastewater management, is a concern, and wastewater management systems have been created to address it [7–8]. The construction sector consumes massive amounts of raw materials. Annual concrete manufacturing, for example, necessitates 20 billion tons of aggregate, 1.5 billion tons of cement, and 0.8 billion tons of water globally [6]. According to [9], the most significant feature of concrete is strength, which affects the quality of the concrete . Engineers and builders utilize the compressive strength of concrete as the most frequent performance metric for testing concrete cubes and cylinders. The failure load divided by the cross-sectional area resisting the load yields the compressive strength of concrete.

The following measures should be followed by ready-mixed concrete production plants in order to adhere to the principles of sustainable building [10-13].

1- decreasing natural resource and energy use,

- 1- decreasing natural resource and energy use,
- 2- recovering and reuse products (recycling),

3- have used energy from the environment resources and renewable sources.

#### **2. Materials and Methods 2.1 Cement**

Ordinary Portland Cement (OPC) Type 1 produced in Sulaymaniyah; Iraq was utilized in this investigation (Tasluja). This is the most common type of cement used in Iraq's building sector. The cement is packaged in paper and plastic bags in accordance with Iraqi Standard Specifications (No. 5:1984). The most essential cement qualities are shown in Tables 1 and 2.

<b>Compound Composition</b>	By weight %	Limits $\circ$ of Iraqi				
		Specification No.5:1984				
Lime (CaO)	61.30					
Silica (SiO2)	19.55					
Iron oxide (Fe2O3)	3.99					
Alumina (Al2O3)	4.97					
Magnesia oxide (MgO)	2.2	< 5.0				
Sulfate (SO3)	1.13	2.8				
Loss on Ignition	1.39	4.0				
Lime saturation factor	0.87	$0.66 - 1.02$				
Insoluble residue	0.99	< 1, 5				
Main Compounds (Bogue's equation) % by weight of cemen						
Tricalcium silicate (C3S)	48.33					
Dicalcium silicate (C2S)	32.39					
Tricalcium aluminate	4.77					
(C3A)						
Tetracalcium	4.87					
aluminoferrite (C4AF)						

**Table 1:** Chemical Composition of Cement

Table 2: Physical Properties of Cement

Physical properties	Test result	Limits of Iraqi specification No. 5:1984	
surface Specific area, Fineness Blaine method (m2 / kg)	262.5	>230	
Setting time by Vicat's			
method Initial setting(min)	166	>45	
Final setting(min)	245	$<$ 10 hrs	
Soundness using Auto clave	0.31	< 0.8	
(%)			

#### **2.2 Fine Aggregate**

The study employed typical sand from the AL-Ukhaider region in Karbala, Iraq, which complies with Iraqi Standard Specification (No.45: 1984-Zone II). Sand is a readily available, low-cost commodity that helps to reduce the cost of a concrete mix. Table 3 displays the findings of the fine aggregate sieve study. Table 4 shows the chemical and physical characteristics of fine aggregate



![](_page_2_Picture_307.jpeg)

![](_page_2_Picture_308.jpeg)

![](_page_2_Picture_309.jpeg)

#### **2.3 Coarse Aggregate**

The usage of gravel with a maximum size of 12.5 mm from the AL-Nibaee region (AL Anbar, Iraq). The sieve analysis of natural coarse aggregate

according to Iraqi Standard Specification is shown in Table 5. (No.45: 1984). The physical features of this aggregate are shown in Table 6.

![](_page_2_Picture_310.jpeg)

![](_page_2_Picture_311.jpeg)

**Table 6:** Chemical and Physical characteristics of coarse aggregates

![](_page_2_Picture_312.jpeg)

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![](_page_3_Picture_374.jpeg)

## **2.4 Mixing Water**

Three distinct sources of water were used. 250 feet below the land surface, groundwater or tap water was obtained, while surface water was drawn from Baghdad's water canal. Bicarbonates, conductivity, hardness, total dissolved solids (T.D.S), total suspended solids (T.S.S), dissolved oxygen, pH, biochemical oxygen demand, and chemical oxygen demand were all tested in the water.

**Table 7.** results of the water tests

2.5	Parameters	Tap water	Well water	Surface water	Maximum
					Allowable
					Limit(WHO)
	pH	7.5	7	7.3	$6.5 - 8.5$
	Temp. C <sup>0</sup>	17	17	17	
	T.D.S (mg/l)	420	700	950	1000
	T.S.S (mg/l)	12	50	76	155
	Bicarbonates(mg/l)		290	185	1000
	Cond. $(ms/cm)$	740	1100	1350	1000
	Turbidity (NTU)	0.8	1.5	6	10
	Hardness (mg/l)	54.6	270	120	100
	DO(mg/l)	5.15	6.1	6.5	$4 - 7$
	$COD$ (mg/l)	11.2	13	47	150
	$BOD$ (mg/l)	2	9	29	80

### **Mix Design and Sample Preparation**

For the manufacture of concrete based on a cement, sand, and aggregate combination, one mix design proportions were employed. These ratios were (1:2:4). For both design proportions, the water–cementratio was held constant at 0.60. It should be emphasized that just one water sample was utilized at a time when producing the concrete, and no additional water samples were intermixed in any instance or design ratio. According to the American Society for Testing and Materials, the elements were weighted in a separate tray before being blended in a concrete mixer (ASTM C192-98). The whole mixing time was 5–7 minutes, following which the concrete mix was compressed using a vibrating table. The slump test was performed to measure the workability of the concrete and to compare the effect of the water sample on the workability of the concrete. In addition, the compacting factor test was carried out to ensure that the produced concrete was workable. After 24 hours, the specimens were molded, cured in water, and

tested at room temperature for the requisite period. In the casting process, 150 mm diameter 300 mm long cylinders were made 36 for each mix design (1:2:4) to assess compressive and tensile strength. In addition, 36 100 mm100 mm500 mm prisms or beams were cast to evaluate the flexural strength (modulus of rupture) for each combination. As a result, 72 samples were created (36 (comp strengthcylinder) + 36 (tensile strength-cylinder). After 7, 28 and 90 days of curing, all of these samples were evaluated. After curing, the following tests were performed on the concrete specimens: A compressive strength test was performed at 7, 28, and 90 days according to ASTM C39, with a loading rate of 2.5 kN/s;•A splitting cylinder tensile test was performed at 7, 28, and 90 days according to ASTM C496-96, with an increasing loading rate of 2 kN/s;•A three-point loaded, flexure strength test of a beam was performed according to ASTMC [10,14].

#### **3 Results Compressive Strength and Splitting Tests**

The compressive strength, tensile strength, and lifespan of concrete created with well water, power station water, and fresh water are

graphed to help explain the differences in compressive strength and splitting with ages. This is demonstrated in Figure 1 and 2 and Table 8.

![](_page_4_Picture_371.jpeg)

![](_page_4_Figure_6.jpeg)

![](_page_4_Figure_7.jpeg)

![](_page_4_Figure_8.jpeg)

**Figure 2.** Graph of splitting against Age of Concrete

Table 8 and Figures 1 and 2 demonstrate that the compressive strength and splitting of concrete cubes created using tap water and surface water improved considerably with age, indicating that tap water and surface water are suitable for use in concrete manufacturing.

There was a gradual increase in compressive strength and tensile strength from (18.5 and 16.5) and (1.5 and 1.3) N/mm2 to (28 and 24) and (3 and 2.6) N/mm2, and it was discovered from Table 8 and Figure 1 and 2, that the compressive strength and tensile strength of well water-produced concrete cubes tended to increase from 28 days, However, with a 90-day age, it is quite little. The presence of components such as Na, K, Ca, and Cl aided in increasing the rate of hydration, allowing for an early increase in compressive strength but afterwards experiencing a sudden constant due to their decrease quantities.

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